Special Flood Hazard Evaluation Report

Chagrin River and Aurora Branch

Village of Bentleyville, Cuyahoga County, Ohio

Prepared for the Ohio State Department of Natural Resources



US Army Corps of Engineers Buffalo District

March 2000

20000419 075

REPORT DOCUMENTATION PAGE

Form Approved OMB No. 0704-0188

Public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing this collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden to Department of Defense, Washington Headquarters Services, Directorate for Information Operations and Reports (0704-0188), 1215 Jefferson Davis Highway, Suite 1204, Arlington, VA 22202-4302. Respondents should be aware that notwithstanding any other provision of law, no person shall be subject to any penalty for failing to comply with a collection of information if it does not display a currently valid OMB control number. PLEASE DO NOT RETURN YOUR FORM TO THE ABOVE ADDRESS.

1. REPORT DATE (DD-MM-YYYY)	2. REPORT TYPE	3. DATES COVERED (From - To)
03/01/2000	Final	
4. TITLE AND SUBTITLE		5a. CONTRACT NUMBER
Special Flood Hazard Eva		
Aurora Branch, Village of I	5b. GRANT NUMBER	
	5c. PROGRAM ELEMENT NUMBER	
6. AUTHOR(S)		5d. PROJECT NUMBER
·		5e. TASK NUMBER
	5f. WORK UNIT NUMBER	
	A REDECRIMING ORGANIZATION DEPORT	
7. PERFORMING ORGANIZATION NAME(S) AND ADDRESS(ES)	8. PERFORMING ORGANIZATION REPORT NUMBER
U.S. Army Engineer Distric		
U.S. Army Engineer Distric Buffalo		
U.S. Army Engineer Distric Buffalo 1776 Niagara Street	t,	
U.S. Army Engineer Distric Buffalo	t,	
U.S. Army Engineer Distric Buffalo 1776 Niagara Street Buffalo, New York 14207-3	t, 199	
U.S. Army Engineer Distric Buffalo 1776 Niagara Street Buffalo, New York 14207-3	t, 199 (NAME(S) AND ADDRESS(ES)	NUMBER
U.S. Army Engineer District Buffalo 1776 Niagara Street Buffalo, New York 14207-3 9. SPONSORING / MONITORING AGENCY Ohio Department of Natural	t, 199 (NAME(S) AND ADDRESS(ES)	NUMBER 10. SPONSOR/MONITOR'S ACRONYM(S)
U.S. Army Engineer Distric Buffalo 1776 Niagara Street Buffalo, New York 14207-3	t, 199 (NAME(S) AND ADDRESS(ES)	NUMBER 10. SPONSOR/MONITOR'S ACRONYM(S)

Approved for Public Release; Distribution Unlimited

13. SUPPLEMENTARY NOTES

14. ABSTRACT

This report documents the results of an investigation to determine the potential flood Situation along the Chagrin River and Aurora Branch within the Village of Bentleyville, Cuyahoga County, Ohio. The study reach includes the Chagrin River from the northern corporate boundary, upstream, to the eastern corporate boundary. The Aurora Branch was studied from its confluence with the Chagrin River to the southern corporate boundary. Consideration of a rock ledge located approximately 9600 feet downstream of the confluence of the Chagrin River with the Aurora Branch in this study produced significant increases in flood elevations for the Chagrin River. In addition, the current study modeled the Aurora Branch for its entire length within the Village of Bentleyville therefore producing more accurate results. Findings from this study may warrant reexamining the adjoining upstream study area because flood elevations between the two studies are not in concurrence.

flood plain floods		-	rin River age of Bentleyville							
16. SEODRITY CLAS	SIFICATION OF:		17. LIMITATION OF ABSTRACT	18. NUMBER OF PAGES	19a. NAME OF RESPONSIBLE PERSON Edward Gustek					
a.REPORT Unclassified	b. ABSTRACT Unclassified	c. THIS PAGE Unclassified	UL	23	19b. TELEPHONE NUMBER (include area code) 716-879-4143					

SPECIAL FLOOD HAZARD EVALUATION REPORT CHAGRIN RIVER AND AURORA BRANCH VILLAGE OF BENTLEYVILLE, CUYAHOGA COUNTY, OHIO

TABLE OF CONTENTS

Description

INTRODUCTION								
PRINCIPAL FLOOD PROBLEMS	3							
Flood Magnitudes and Their Frequencies Hazards and Damages of Large Floods								
HYDROLOGIC ANALYSES	3							
HYDRAULIC ANALYSES	4							
UNIFIED FLOOD PLAIN MANAGEMENT	5							
Modify Susceptibility to Flood Damage and Disruption a. Flood Plain Regulations b. Development Zones c. Formulation of Flood Plain Regulations	7 7 8 8							
Modify Flooding	9							
Modify the Impact of Flooding on Individuals and the Community	9							
CONCLUSION	9							
GLOSSARY	11							
REFRENCES	13							

TABLES

<u>Number</u>		<u>Page</u>
1	Summary of Discharges	4
2	Manning's "n" and Contraction and Expansion Coefficients	5
3	Floodway Data	6
4	Tabular Cross-sectional Data	End of Report
	FIGURES	
1	Vicinity Map	2
2	Floodway Schematic	8
	PLATES	
	Flood Profile, Big Chagrin River (3 sheets)	
	Flood Profile, Aurora Branch (5 sheets)	
	MAPS	
	Flooded Area Map, Chagrin River and Aurora Branch (1 sheet)	

SPECIAL FLOOD HAZARD EVALUATION REPORT CHAGRIN RIVER AND AURORA BRANCH VILLAGE OF BENTLEYVILLE, CUYAHOGA COUNTY, OH

INTRODUCTION

This Special Flood Hazard Evaluation Report documents the results of an investigation to determine the potential flood situation along the Chagrin River and Aurora Branch within the Village of Bentleyville, Cuyahoga County, Ohio. This study was conducted at the request of the Ohio Department of Natural Resources under the authority of Section 206 of the 1960 Flood Control Act, as amended. The study reach includes the Chagrin River from the northern corporate boundary, upstream, to the eastern corporate boundary. The Aurora Branch was studied from its confluence with the Chagrin River to the southern corporate boundary.

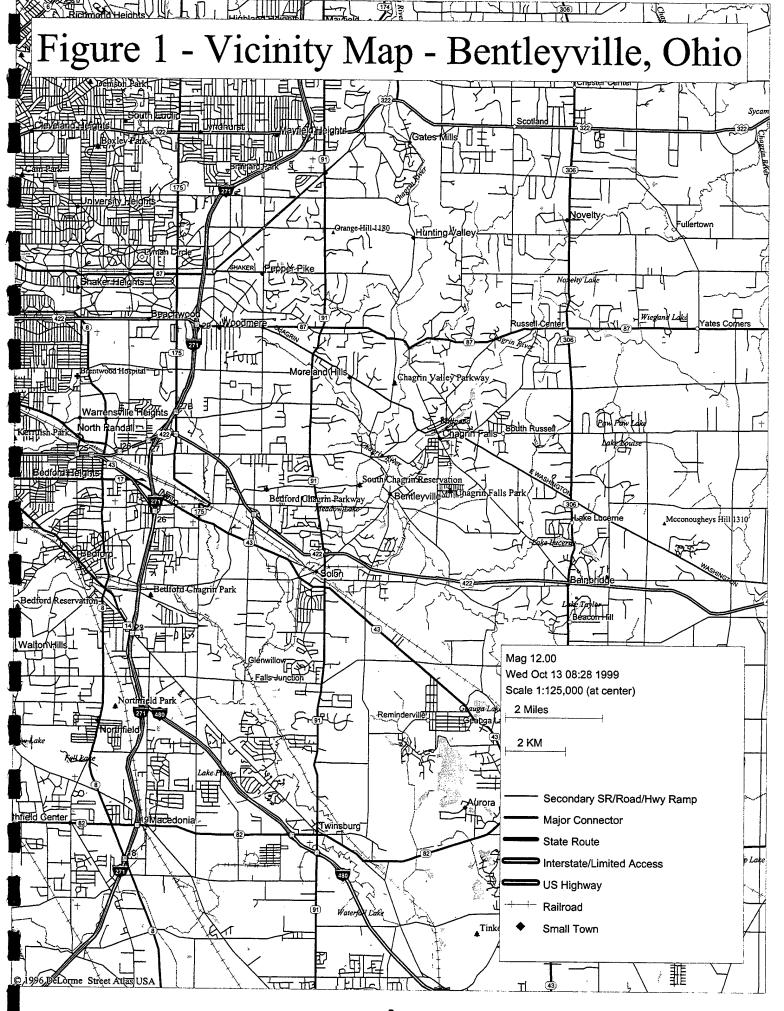
The Village of Bentleyville is located in northeastern Cuyahoga County, approximately 21 miles southeast of Cleveland and 58 miles northwest of Youngstown. The Bentleyville population is 674 according to the 1990 Census (Reference 1). The Chagrin River originates in the Town of Chardon and flows in a southwest direction before turning north and flowing through the Village of Bentleyville to Lake Erie.

Knowledge of potential floods and flood hazards is important in land use planning. This report identifies the 100-year and 500-year flood plains for the reaches studied.

Information developed for this study will be used by local officials to manage future flood plain development. While the report does not provide solutions to flood problems, it does furnish a suitable basis for the adoption of land use controls to guide flood plain development, thereby preventing intensification of the flood loss problem. It will also aid in the development of other flood damage reduction techniques to modify flooding and reduce flood damages which might be embodied in an overall Flood Plain Management (FPM) program. Other types of studies, such as environmental and current and future land use analysis of the flood plain, would also profit from this information.

Although Flood Insurance Rate Maps have been developed for the community, recent flooding events indicate that changes may have occurred which effect the flood plain. Currently, the area is now experiencing residential development pressure, and local officials requested detailed flood plain information to assist them in managing development.

Additional copies of this report can be obtained from the Ohio State Department of Natural Resources until its supply is exhausted, and the National Technical Information Service of the U.S. Department of Commerce, Springfield, Virginia 22161, at the cost of reproducing the report. The Buffalo District Corps of Engineers will provide technical assistance and guidance to planning agencies in the interpretation and use of the hydrologic data obtained for this study.



PRINCIPAL FLOOD PROBLEMS

Although flooding may occur during any season, the principal flood problems have occurred during winter and spring months and are usually the result of spring rains and or snowmelt.

Flood Magnitudes and Their Frequencies

Special flood hazard areas are determined with reference to the "100-year" flood standard, which is a national standard on which flood regulations are based. The "100-year" flood, also referred to as a "base flood," is defined as the flood having a 1 percent probability of being equaled or exceeded in any given year. The risk of experiencing a flood of this magnitude increases with the length of time considered. While it represents the long-term average recurrence interval for a flood of this magnitude, such floods may be experienced in any given year. There is a greater than 50 percent probability that a "100-year" event will occur during a 70-year life time, and there is a 26 percent (about one in four) probability of experiencing such a flood over a typical 30-year mortgage period. The "100-year" flood is more properly termed the 1 percent chance exceedance flood or 1 percent flood, which represents its true probability of being equaled or exceeded in any year.

Hazards and Damages of Large Floods

The extent of damage caused by any flood depends on the topography of the flooded area, the depth and duration of flooding, the velocity of flow, the rate of rise in water surface elevation, and development of the flood plain. Deep water flowing at a high velocity and carrying floating debris would create conditions hazardous to persons and vehicles which attempt to cross the flood plain. Generally, water 3 or more feet deep, which flows at a velocity of 3 or more feet per second, could easily sweep an adult off his feet and create definite danger of injury or drowning. Rapidly rising and swiftly flowing floodwater may trap persons in homes that are ultimately destroyed or in vehicles that are ultimately submerged or floated. Since water lines can be ruptured by deposits of debris and by the force of flood waters, there is the possibility of contaminated domestic water supplies. Damaged sanitary sewer lines and sewage treatment plants could result in the pollution of floodwaters and could create health hazards. Isolation of areas by floodwater could create hazards in terms of medical, fire, or law enforcement emergencies.

HYDROLOGIC ANALYSES

Hydrologic analyses were carried out to determine the peak discharge-frequency relationships for the flooding sources affecting the community. Hydrology was developed for two reaches on the Chagrin River and two reaches on Aurora Branch. For both streams, the peak discharges were calculated using Techniques for Estimating Flood-Peak Discharges of Rural, Unregulated Streams in Ohio developed by the United States Geological Survey (Reference 2). Discharges for the 1 percent (100-year) and 0.2 percent (500-year) floods were analyzed for this study. The 0.2 percent flood peak discharge was extrapolated from the discharge-frequency relationship developed using annual peak discharges of the 50 percent (2-year), 20 percent (5-year), 10 percent (10-year), 4 percent (25-year), 2 percent (50-year), and 1 percent floods. Table 1 summarizes the peak discharges for reaches of the Chagrin River and Aurora Branch.

TABLE 1 SUMMARY OF DISCHARGE

	Drainage	Peak Discharges										
Flooding Source and Location	(sq mi)	10 percent (cfs)	4 percent (cfs)	2 percent (cfs)	1 percent (cfs)	$\frac{0.2 \text{ percent}}{\text{(cfs)}}$						
Chagrin River												
Down stream Bentleyville corporate limit	124	8920	11555	13570	15670	21600						
At confluence with Aurora Branch	61	4890	6325	7425	8565	11700						
Aurora Branch												
At confluence with Chagrin River	58	4670	6035	7080	8160	10130						
Approximately 1450 feet Upstream of Solon Rd.	54	4400	5690	6680	7700	10070						

HYDRAULIC ANALYSES

Analyses of the hydraulic characteristics of flooding from sources studied were carried out to provide estimates of the elevations for the 10 percent, 2 percent, 1 percent and 0.2 percent exceedance floods.

Cross-section data for the backwater analyses of the Chagrin River and Aurora Branch were obtained from field inspections performed by Buffalo District personnel. Additional data were obtained from detailed topographic maps provided by the Cuyahoga County Engineer (Reference 3) and from the previous Village of Bentleyville Flood Insurance Study (Reference 4). Plan diagrams, provided by the Village of Bentleyville, and the existing Flood Insurance Study were used to determine elevation and structural geometry of bridges, culverts, and the rock ledge approximately 9600 feet downstream of the confluence with the Aurora Branch.

Water surface elevations of the 1 percent and 0.2 percent flood events were computed using the COE HEC-RAS step-backwater computer program (Reference 5). Starting water surface elevations for the Chagrin River and Aurora Branch were taken from the Village of Bentleyville Flood Insurance Study dated August 1980.

Locations of the selected cross-sections used in the hydraulic analyses are shown on the Flood Profile (Plate 1) and on the Flooded Areas Map which accompany this report.

Channel and overbank roughness factors (Manning's "n") used in the hydraulic computations were selected using engineering judgement and were based on field observations of the stream and flood plain areas. The values for Manning's "n" and the contraction and expansion coefficients are shown in Table 2.

TABLE 2 MANNING'S "N" AND CONTRACTION & EXPANSION COEFFICIENTS

Flooding Source	<u>Channel</u>	<u>Overbank</u>	Contraction	Expansion
Chagrin River	0.03-0.035	0.08	0.1	0.3
Aurora Branch	0.03-0.045	0.05-0.055	0.1-0.3	0.3-0.5

Flood profiles were drawn showing the computed water surface elevations for the selected recurrence intervals. The flood plain boundaries were delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated using the topographic maps and spot elevations obtained during field inspection. Small areas within the flood plain boundaries may be above the flood elevations, but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

Floodways were determined for the streams studied in detail. Floodway encroachments were based on equal conveyance reduction from each side of the flood plain, with adjustments as necessary to provide functional and manageable floodways. At the request of the ODNR, the maximum increase in stage due to encroachment was limited to 1 foot, provided that hazardous velocities were not produced. Floodway widths were computed at cross sections for the Chagrin River and for Aurora Branch. Between cross sections, the floodway boundaries were interpolated. The results of the floodway calculations are summarized for selected cross sections in Table 3.

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the profile are considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

The current study did not identify new elevation reference marks or modify elevation reference marks from the previous study.

The previously mentioned flood events may have resulted from isolated storms or other intangibles. A comparison of this study with the 1980 FIS indicates a significant increase in flood elevations for the study area. The 1 percent flood elevations of this study do not concur within 1 foot of those reported for the upstream community (Village of Chagrin Falls). The 1 percent flood elevation reported for the upstream corporate limit of the Village of Bentleyville in this study is approximately 2 feet higher than that reported for the downstream limit of the Village of Chagrin Falls 1980 FIS.

UNIFIED FLOOD PLAIN MANAGEMENT

Historically, the alleviation of flood damage has been accomplished almost exclusively by the construction of protective works such as reservoirs, channel improvements, and floodwalls and levees. However, in spite of the billions of dollars that have already been spent for construction of well-designed and efficient flood control works, annual flood damages continue to increase because the number of persons and structures occupying floodprone lands is increasing faster than protective works can be provided.

,																							
INCREASE (FEET)		9.0	0.7	1.0	0.0	0.0	0.7	6.0	0.8	1.0			0.0	1.0	0.0	0.1	0.2	0.7	0.8	0.7	1.0	6.0	
WITH FLOODWAY (NGVD)		825.8	825.9	827.9	829.3	830.9	835.4	836.8	836.5	839.2			836.4	842.7	853.9	864.4	889.2	890.3	891.2	892.0	893.5	894.5	
WITHOUT FLOODWAY (NGVD)		825.2	825.2	826.9	829.3	830.9	834.7	835.9	835.7	838.2			836.4	841.7	853.9	864.3	0.688	9.688	890.4	891.3	892.5	893.6	
REGULATORY (NGVD)		825.2	825.2	826.9	829.3	830.9	834.7	835.9	835.7	838.2			836.4	841.7	853.9	864.3	889.0	9.688	890.4	891.3	892.5	893.6	
MEAN VELOCITY (FEET/SEC.)		8.6	6.6	11.3	8.5	4.2	4.9	1.9	6.9	4.9			14.9	11.4	14.3	14.0	7.9	5.4	2.6	2.7	5.9	3.4	
SECTION AREA (SQ. FEET)		1592	1577	1386	1847	3748	1765	4577	1240	1762			547	719	571	584	1030	1514	2937	2890	1313	2246	
WIDTH (FEET)		149	152	213	260	559	163	460	166	256			77	06	91	96	06	130	370	480	240	400	
DISTANCE		619	859	1383	1691	2200	3781	5155	5981	68371			1916 ²	28882	38912	48462	6340^{2}	7035^{2}	89432	10477^{2}	128152	13200^{2}	
CROSS SECTION	CHAGRIN RIVER	A	8	ບ	Ω	ഥ	ш	Ü	H	_		AURORA BRANCH	Y	В	ບ	Ω	ш	ĬL,	ט	Н			
	DISTANCE (FEET) (SQ. FEET) (SQ. FEET) (SQ. FEET) (SQ. FEET) (SQ. FEET) (FEET/SEC.)	DISTANCE (FEET) (SQ. FEET) (FEET/SEC.) (NGVD) (NGVD) (NGVD)	DISTANCE (FEET) (SQ. FEET) (FEET/SEC.) (NGVD) (NGVD) (NGVD) (NGVD) (NGVD) (NGVD)	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 825.2	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 825.9 1383 ¹ 213 1386 11.3 826.9 826.9 827.9	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) WITH (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 825.9 1383 ¹ 213 1386 11.3 826.9 825.9 827.9 1697 ¹ 260 1847 8.5 829.3 829.3 829.3	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619¹ 149 1592 9.8 825.2 825.2 825.8 658¹ 152 1577 9.9 825.2 825.2 825.9 1383¹ 213 1386 11.3 826.9 825.2 825.9 1697¹ 260 1847 8.5 829.3 829.3 829.3 2200¹ 559 3748 4.2 830.9 830.9 830.9	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619¹ 149 1592 9.8 825.2 825.2 825.8 658¹ 152 1577 9.9 825.2 825.2 825.9 1383¹ 213 1386 11.3 826.9 826.9 825.9 1697¹ 260 1847 8.5 829.3 829.3 829.3 2200¹ 559 3748 4.2 830.9 830.9 830.9 3781¹ 163 1765 4.9 834.7 834.7 835.4	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 825.9 1383 ¹ 213 1386 11.3 826.9 826.9 825.9 1697 ¹ 260 1847 8.5 826.9 826.9 825.9 1697 ¹ 260 1847 8.5 826.9 826.9 829.3 2200 ¹ 559 3748 4.2 830.9 830.9 830.9 3781 ¹ 163 1765 4.9 835.9 835.9 836.8 5155 ¹ 460 4577 1.9 835.9 835.9 836.8	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 825.9 1897 ¹ 260 1847 8.5 826.9 826.9 826.9 829.3 1697 ¹ 260 1847 8.5 829.3 829.3 829.3 2200 ¹ 559 3748 4.2 830.9 830.9 830.9 3781 ¹ 163 1765 4.9 835.9 835.9 836.8 5155 ¹ 460 4577 1.9 835.7 835.7 836.5 5981 ¹ 166 1240 6.9 835.7 835.7 836.5	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 825.9 1383 ¹ 213 1386 11.3 826.9 826.9 825.9 1697 ¹ 260 1847 8.5 829.3 829.3 829.3 2200 ¹ 559 3748 4.2 830.9 830.9 830.9 3781 ¹ 163 1765 4.9 835.9 835.9 836.8 5155 ¹ 460 4577 1.9 835.9 835.9 836.5 5981 ¹ 166 1240 6.9 838.7 835.9 836.5 6837 ¹ 256 1762 4.9 838.2 838.2 839.2 839.2 838.2 838.2 838.2 839.2 839.2	DISTANCE WIDTH (FEET) SECTION AREA (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITH (NGVD) WITH (NGVD) 619 ¹ 1383 ¹ 1383 ¹ 2200 ¹ 558 3781 ¹ 165 559 3748 460 4577 166 5981 ¹ 166 1240 6837 ¹ 256 1592 825.2 825.2 825.2 825.3 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 826.9 836.9 836.9 836.9 836.9 836.8 836.5 836.	DISTANCE WIDTH (FBET) SECTION (SQ. FBET) MEAN VELOCITY (FBET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITH (NGVD) 619 ¹ 149 1592 9.8 825.2 825.2 825.8 825.9 658 ¹ 152 1577 9.9 825.2 825.9 825.9 825.9 1383 ¹ 213 1386 11.3 826.9 825.9 825.9 825.9 1383 ¹ 163 1765 4.9 835.9 820.3 830.9 830.9 3781 ¹ 166 1240 6.9 835.9 835.9 836.8 5981 ¹ 166 1240 6.9 835.7 835.7 836.5 6837 ¹ 256 1762 4.9 838.2 838.2 839.2	DISTANCE WIDTH (FBET) SECTION (FBET) MEAN (FBET) REGULATORY (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) 619 ¹ 149 1592 9.8 658 ¹ 152 1577 9.9 825.2 825.2 825.3 138 ² 157 9.9 825.2 825.2 825.3 825.9 1383 ¹ 213 1386 11.3 826.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 826.9 827.9 826.9 827.9 826.9 827.9 826.9 826.9 827.9 826.9 8	DISTANCE WIDTH (FEET) SECTION (SQ. FEET) MEAN (FEET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) 619 ¹ 149 1592 9.8 825.2 825.8 658 ¹ 152 1577 9.9 825.2 825.2 183 ¹ 213 1386 11.3 826.9 825.9 1697 ¹ 260 1847 8.5 829.3 829.3 829.3 2200 ¹ 559 3748 4.2 830.9 830.9 830.9 5155 ¹ 460 4577 1.9 834.7 835.9 836.8 5981 ¹ 166 1240 6.9 835.7 835.9 835.8 6837 ¹ 256 1762 4.9 836.4 836.4 836.4 2888 ² 90 719 11.4 841.7 841.7 842.7	DISTANCE WIDTH SECTION MEAN REGULATORY WITHOUT WITH	DISTANCE WIDTH SECTION MEAN REGULATORY FLOODWAY FLOO	MIDTH SECTION MEAN REGULATORY WITHOUT WITH	MIDTH SECTION MEAN REGULATORY WITHOUT WITH	MEGULATORY MEGN MEGN MITHOUT MITHOUT	DISTANCE WIDTH (FBET) SECTION (REET) MEAN (REET/SEC.) REGULATORY (NGVD) WITHOUT (NGVD) WITHOUT (NGVD) 619 ¹ 1383 ¹ 1384 149 152 152 152 153 1384 1592 157 159 159 1607 ¹ 159 1607 ¹ 159 1607 ¹ 150 1607 ¹ 150 1607 ¹ 150 1607 ¹ 150 1607 ¹ 150 1607 ¹ 150 1607 ¹ 160 176 176 176 176 176 176 176 176 176 176	MIDTH SECTION MEAN REGULATORY WITHOUT WITH	Post Control

¹FEET ABOVE CORPORATE LIMIT ²FEET ABOVE MOUTH

Recognition of this trend has forced a reassessment of the flood control concept and resulted in the broadened concept of unified flood plain management programs. Legislative and administrative policies frequently cite two approaches: structural and nonstructural, for adjusting to the flood hazard. In this context, "structural" is usually intended to mean adjustments that modify the behavior of floodwaters through the use of measures such as dams and channel work. "Nonstructural" is usually intended to include all other adjustments in the way society acts when occupying or modifying a flood plain (e.g., regulations, floodproofing, insurance, etc.). Both structural and nonstructural tools are used for achieving desired future flood plain conditions. There are three basic strategies which may be applied individually or in combination: (1) modifying the susceptibility to flood damage and disruption, (2) modifying the floods themselves, and (3) modifying (reducing) the adverse impacts of floods on the individual and the community.

Modify Susceptibility to Flood Damage and Disruption

The strategy to modify susceptibility to flood damage and disruption consists of actions to avoid dangerous, economically undesirable, or unwise use of the flood plain. Responsibility for implementing such actions rests largely with the non-Federal sector and primarily at the local level of government.

These actions include restrictions in the mode and the time of occupancy; in the ways and means of access; in the pattern, density, and elevation of structures and in the character of their materials (structural strength, adsorptiveness, solubility, corrodibility); in the shape and type of buildings and in their contents; and in the appurtenant facilities and landscaping of the grounds. The strategy may also necessitate changes in the interdependencies between flood plains and surrounding areas not subject to flooding, especially interdependencies regarding utilities and commerce. Implementing mechanisms for these actions include land use regulations, development and redevelopment policies, floodproofing, disaster preparedness and response plans, and flood forecasting and warning systems.

Different tools may be more suitable for developed or underdeveloped flood plain or for urban or rural areas. The information contained in this report is particularly useful for the preparation of flood plain regulations.

Flood Plain Regulations.

Flood plain regulations apply to the full range of ordinances and other means designed to control land use and construction within floodprone areas. The term encompasses zoning ordinances, subdivision regulations, building and housing codes, encroachment line statutes, open area regulations, and other similar methods of management which affect the use and development of floodprone areas.

Flood plain land use management does not prohibit use of floodprone areas; to the contrary, flood plain land use management seeks the best use of flood plain lands. The flooded area maps and the water surface profiles contained in this report can be used to guide development in the flood plain. The elevations shown on the profile should be used to determine flood heights because they are more accurate than the outlines of flooded areas. It is recommended that development in areas susceptible to frequent flooding adhere to the principles expressed in Executive Order 11988 - Flood Plain Management, whose objective is to "... avoid to the extent possible the long- and short-term adverse impacts associated with the occupancy and modification of flood plains ... whenever there is a practicable

alternative." Accordingly, development in areas susceptible to frequent flooding should consist of construction which has a low damage potential such as parking areas, parks, and golf courses. High value construction such as buildings, should be located outside the flood plain to the fullest extent possible. In instances where no practicable alternative exists, the land should be elevated to minimize damages. If it is uneconomical to elevate the land in these areas, means of floodproofing the structure should be given careful consideration.

b. Development Zones.

A flood plain consists of two zones. The first zone is the designated "floodway" or that cross sectional area required for carrying or discharging the anticipated flood waters with a maximum 1-foot increase in flood level (New York State Department of Environmental Conservation standard). Velocities are the greatest and most damaging in the floodway. Regulations essentially maintain the flow-conveying capability of the floodway to minimize inundation of additional adjacent areas. Uses which are acceptable for floodways include parks, parking areas, open spaces, etc. The second zone of the flood plain is termed the "floodway fringe" or restrictive zone, in which inundation might occur but where depths and velocities are generally low. Although not recommended if practicable alternatives exist, such areas can be developed provided structures are placed high enough or floodproofed to be reasonably free from flood damage during the 1 percent flood. Typical relationships between the floodway and floodway fringe are shown in Figure 2.

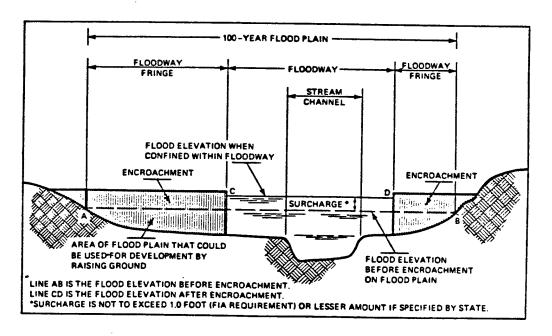


Figure 2 - Floodway Schematic

c. <u>Formulation of Flood Plain Regulations</u>.

Formulation of flood plain regulations in a simplified sense involves selecting the type and degree of control to be exercised for each specific flood plain. In principle, the form of the regulations is not as

important as a maintained adequacy of control. The degree of control normally varies with the flood hazard as measured by depth of inundation, velocity of flow, frequency of flooding, and the need for available land. Considerable planning and research is required for the proper formulation of flood plain regulations. Formulation of flood plain regulations may require a lengthy period of time during which development is likely to occur. In such cases, temporary regulations should be adopted and amended later as necessary.

Modify Flooding

The traditional strategy of modifying floods through the construction of dams, dikes, levees and floodwalls, channel alterations, high flow diversions and spillways, and land treatment measures has repeatedly demonstrated its effectiveness for protecting property and saving lives, and it will continue to be a strategy of flood plain management. However, in the future, reliance solely upon a flood modification strategy is neither possible nor desirable. Although the large capital investment required by flood modifying tools has been provided largely by the Federal government, sufficient funds from Federal sources have not been and are not likely to be available to meet all situations for which flood modifying measures would be both effective and economically feasible. Another consideration is that the cost of maintaining and operating flood control structures falls upon local governments.

Flood modifications acting alone leave a residual flood loss potential and can encourage an unwarranted sense of security leading to inappropriate use of lands in the areas that are directly protected or in adjacent areas. For this reason, measures to modify possible floods should usually be accompanied by measures to modify the susceptibility to flood damage, particularly by land use regulations.

Modify the Impact of Flooding on Individuals and the Community

A third strategy for mitigating flood losses consists of actions designed to assist individuals and communities in their preparatory, survival, and recovery responses to floods. Tools include information dissemination and education, arrangements for spreading the costs of the loss over time, purposeful transfer of some of the individual's loss to the community by reducing taxes in flood prone areas, and the purchase of Federally subsidized flood insurance.

The distinction between a reasonable and unreasonable transfer of costs from the individual to the community can also be regulated and is a key to effective flood plain management.

CONCLUSION

Changes in hydrology and hydraulics reflected in this study are the result of a more detailed analysis. New modeling procedures and additional data have been used in the analysis of the Chagrin River and the Aurora Branch. Consideration of a rock ledge located approximately 9600 feet downstream of the confluence of the Chagrin River with the Aurora Branch in this study produced significant increases in flood elevations for the Chagrin River. In addition, the current study modeled the Aurora Branch for its entire length within the Village of Bentleyville therefore producing more accurate results. Findings from this study may warrant reexamining the adjoining upstream study area because flood elevations between the two studies are not in concurrence.

This report presents local flood hazard information for the Chagrin River in the Village of Bentleyville, Ohio. The U.S. Army Corps of Engineers, Buffalo District, will provide interpretation in the application of the data contained in this report, particularly as to its use in developing effective flood plain regulations. Requests should be coordinated with the Ohio State Department of Natural Resources.

GLOSSARY

BACKWATER EFFECT

The resulting rise in water surface in a given stream due to a downstream obstruction or high stages in an intersecting stream.

BASE FLOOD

A flood which has an average return interval in the order of once in 100 years, although the flood may occur in any year. It is based on statistical analysis of streamflow records available for the watershed and analysis of rainfall and runoff characteristics in the general region of the watershed. It is commonly referred to as the "100-year flood."

DISCHARGE

The quantity of flow in a stream at any given time, usually measured in cubic feet per second (cfs).

FLOOD

An overflow of lands not normally covered by water. Floods have two essential characteristics: the inundation of land is temporary and the lands are adjacent to and inundated by overflow from a river, stream, ocean, lake, or other body of standing water.

Normally a "flood" is considered as any temporary rise in streamflow or stage, but not the ponding of surface water, that results in significant adverse effects in the vicinity. Adverse effects may include damages from overflow of land areas, temporary backwater effects in sewers and local drainage channels, creation of unsanitary conditions or other unfavorable situations by deposition of materials in stream channels during flood recessions, and rise of groundwater coincident with increased streamflow.

FLOOD CREST

The maximum stage or elevation reached by floodwaters at a given location.

FLOOD FREQUENCY

A statistical expression of the percent chance of exceeding a discharge of a given magnitude in any given year. For example, a 100-year flood has a magnitude expected to be exceeded on the average of once every hundred years. Such a flood has a 1 percent chance of being exceeded in any given year. Often used interchangeably with RECURRENCE INTERVAL.

FLOOD PLAIN

The areas adjoining a river, stream, watercourse, ocean, lake, or other body of standing water that have been or may be covered by floodwater.

FLOOD PROFILE

A graph showing the relationship of water surface elevation to location; the latter generally expressed as distance upstream from a known point along the approximate centerline of a stream of water that flows in an open channel. It is generally drawn to show surface elevation for the rest of a specific flood, but may be prepared for conditions at a given time or stage.

FLOOD STAGE

The stage or elevation at which overflow of the natural banks of a stream or body of water begins in the reach or area in which the elevation is measured.

FLOODWAY

The channel of a watercourse and those portions of the adjoining flood plain required to provide for the passage of the selected flood (normally the 100-year flood) with an insignificant increase in the flood levels above that of natural conditions. As used in the National Flood Insurance Program, floodways must

be large enough to pass the 100-year flood without causing an increase in elevation of more than a specified amount (1 foot in most areas).

RECURRENCE INTERVAL

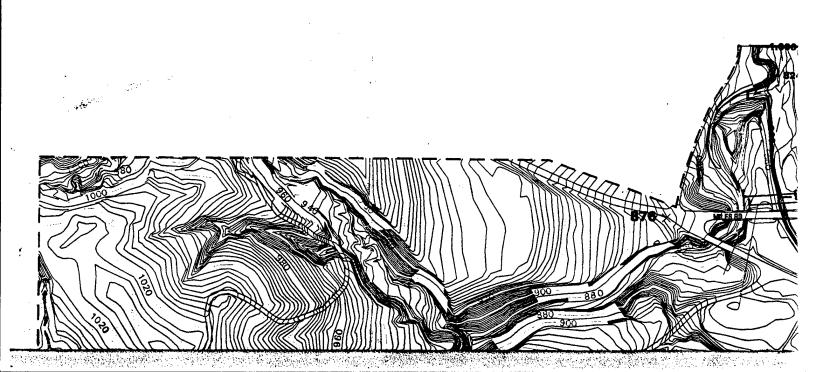
A statistical expression of the average time between floods exceeding a given magnitude (see FLOOD FREQUENCY).

REFERENCES

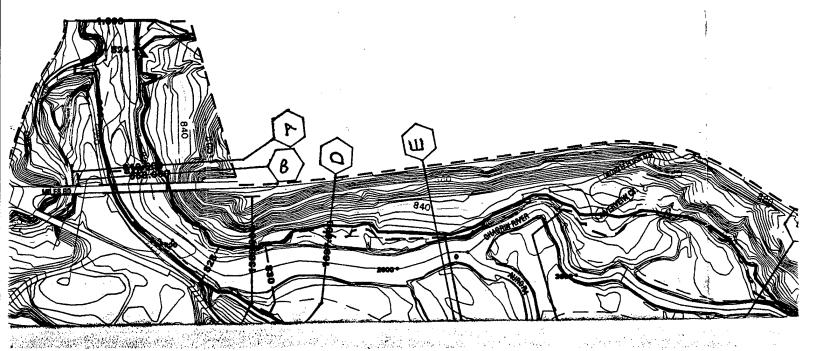
- U.S. Department of Commerce, Bureau of the Census, <u>1990 Census of the Population and Housing</u>, Washington, D.C.
- 2. U.S. Department of the Interior, Geological Survey, <u>Techniques for Estimating Flood-Peak Discharges of Rural, Unregulated Streams In Ohio</u>, Water Resources Investigation Report 89-4126, 1990.
- 3. Cuyahoga County Engineer, <u>Topographic Maps</u>, October 1997
- 4. Federal Emergency Management Agency, <u>Flood Insurance Study</u>, Village of Bentleyville, Ohio, August 1, 1980.
- 5. U.S. Army Corps of Engineers, Hydrologic Engineering Center, <u>HEC-RAS</u>, <u>River Analysis System</u>, Version 2.2, Davis, California, 1988.

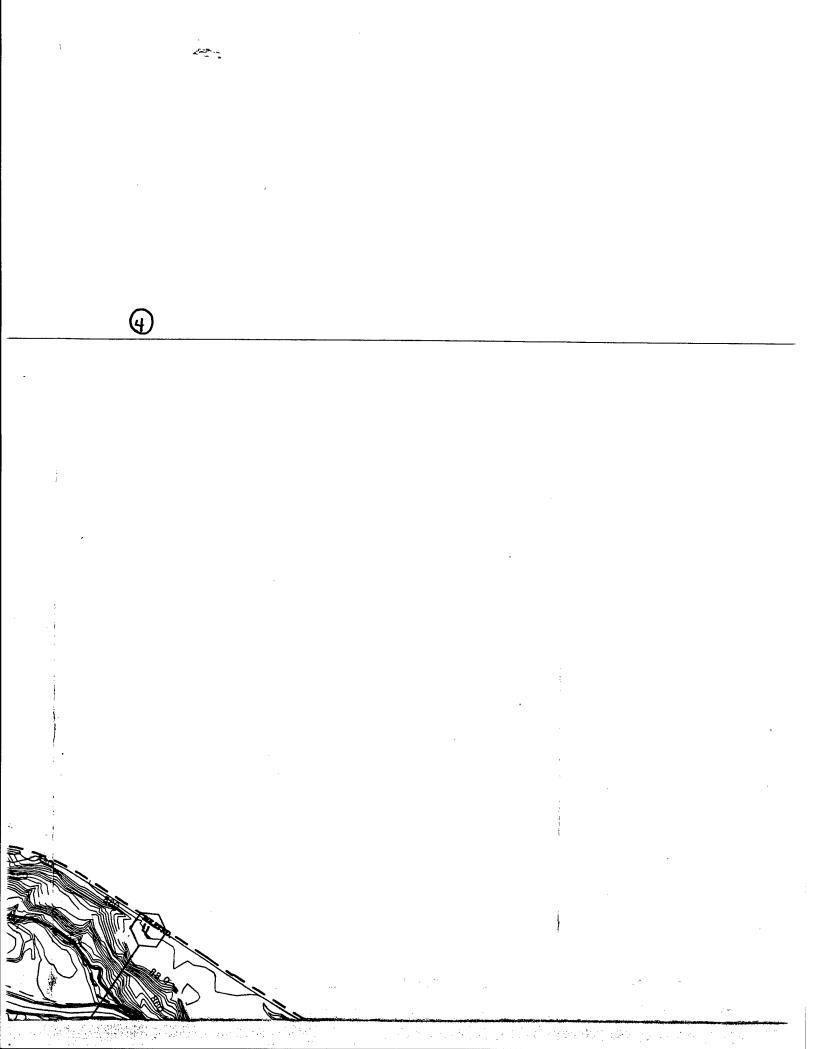


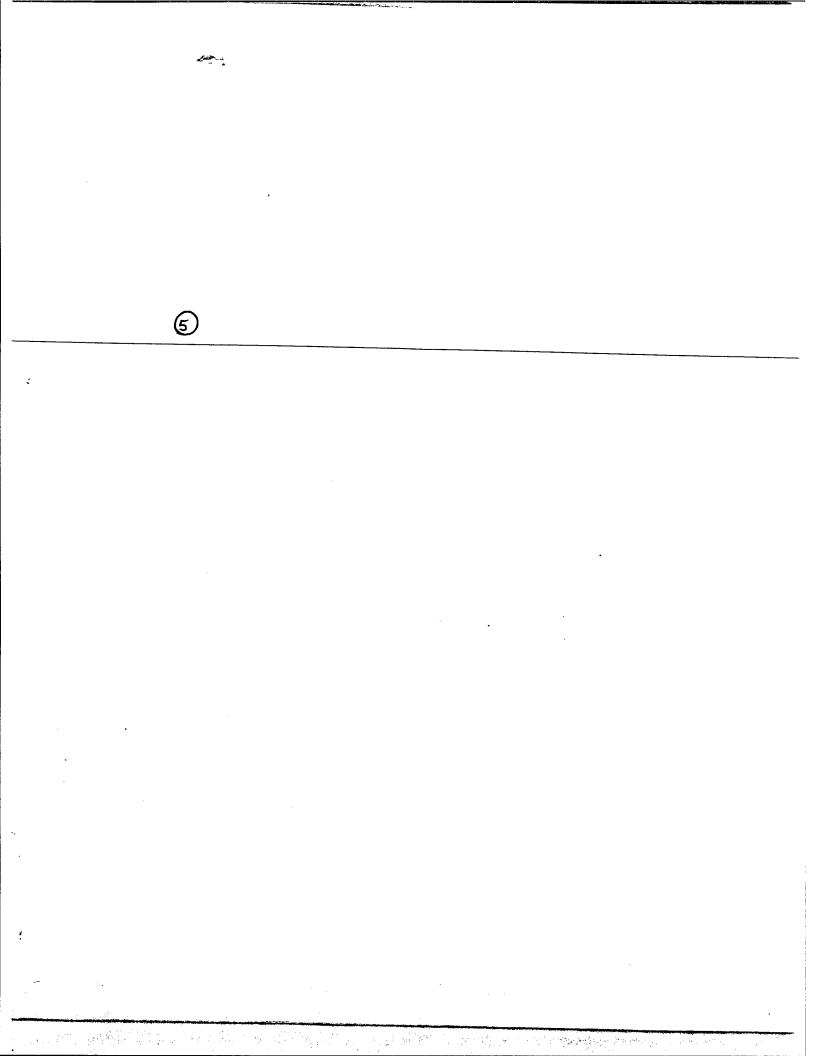






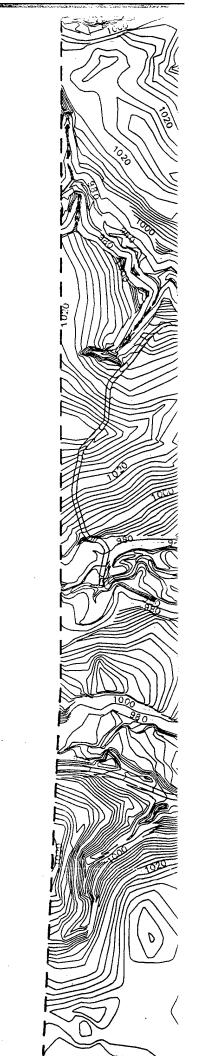


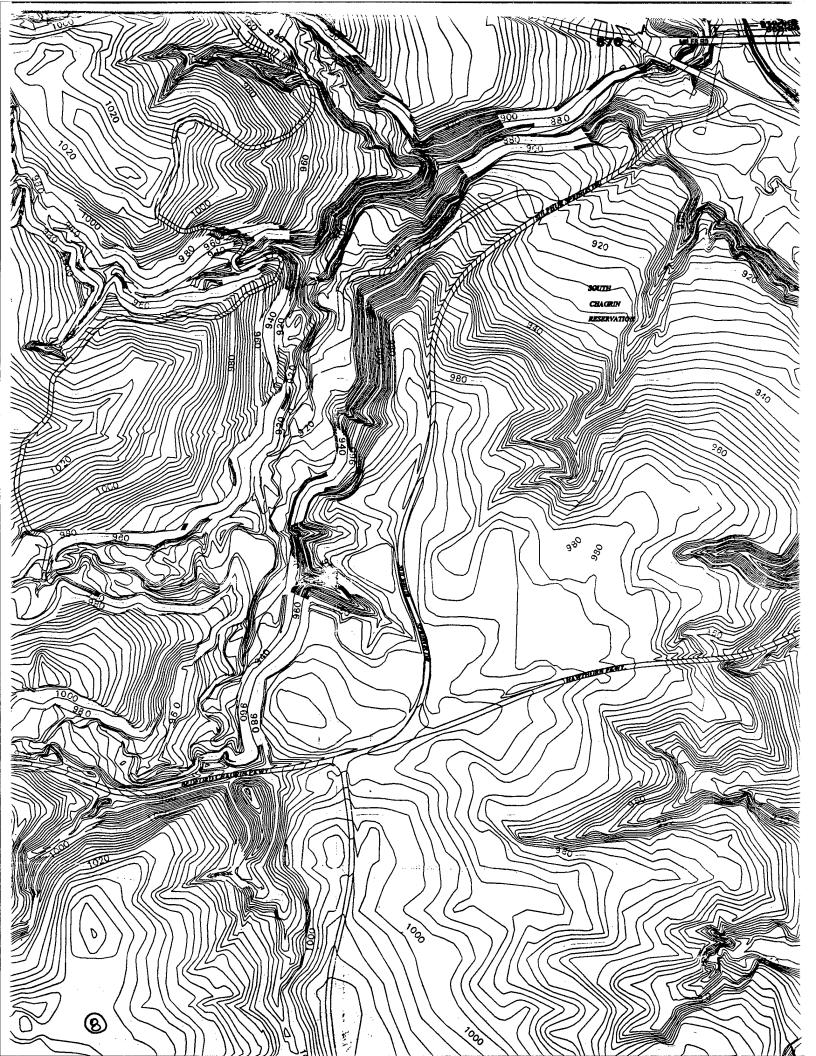


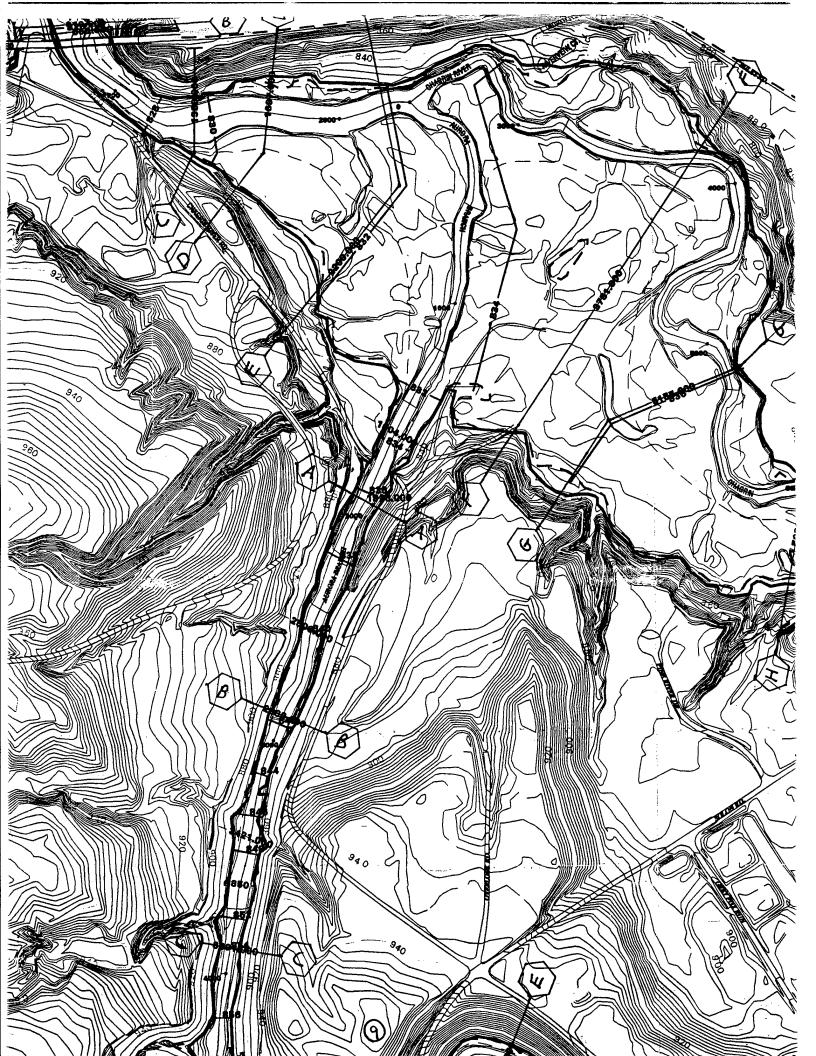


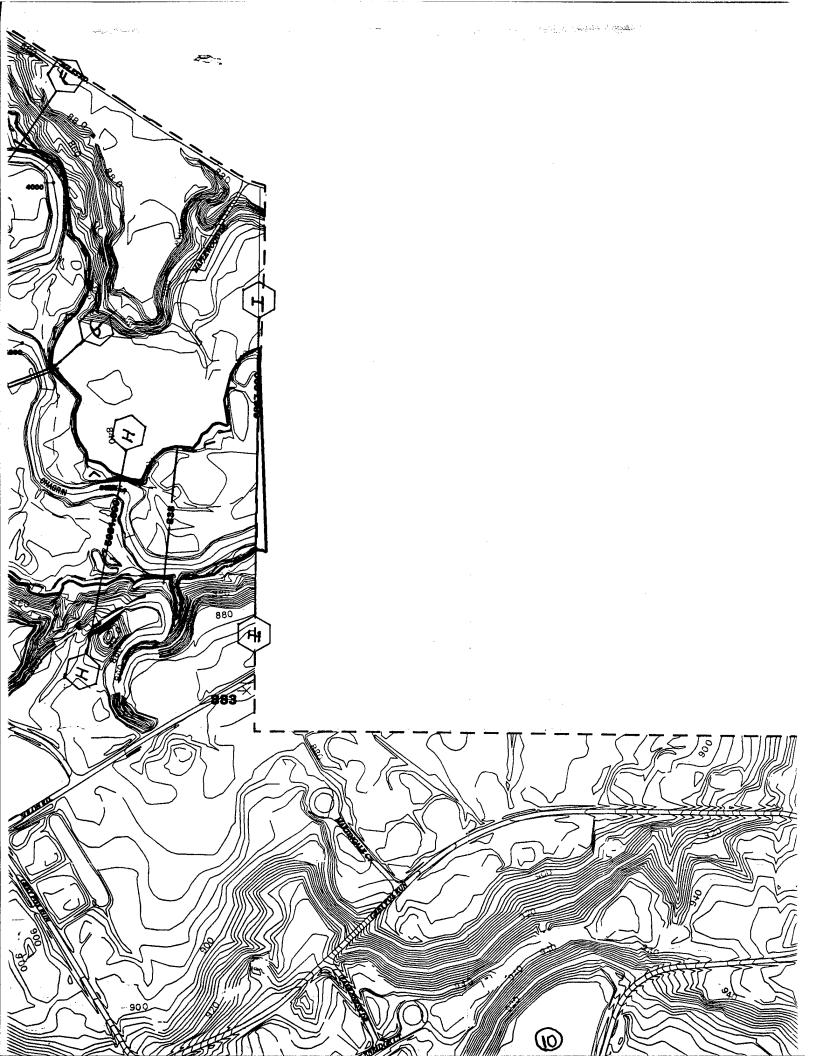
DRAFT PLOT

KEY











KEY



Floodway



100yr Boundary



500yr Boundary



BFE



Study Limits



River Center



River Banks



Cross Section



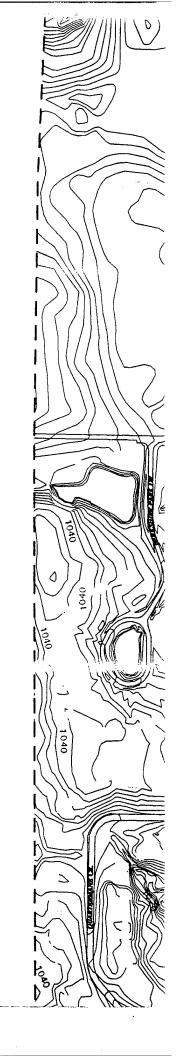
Streets

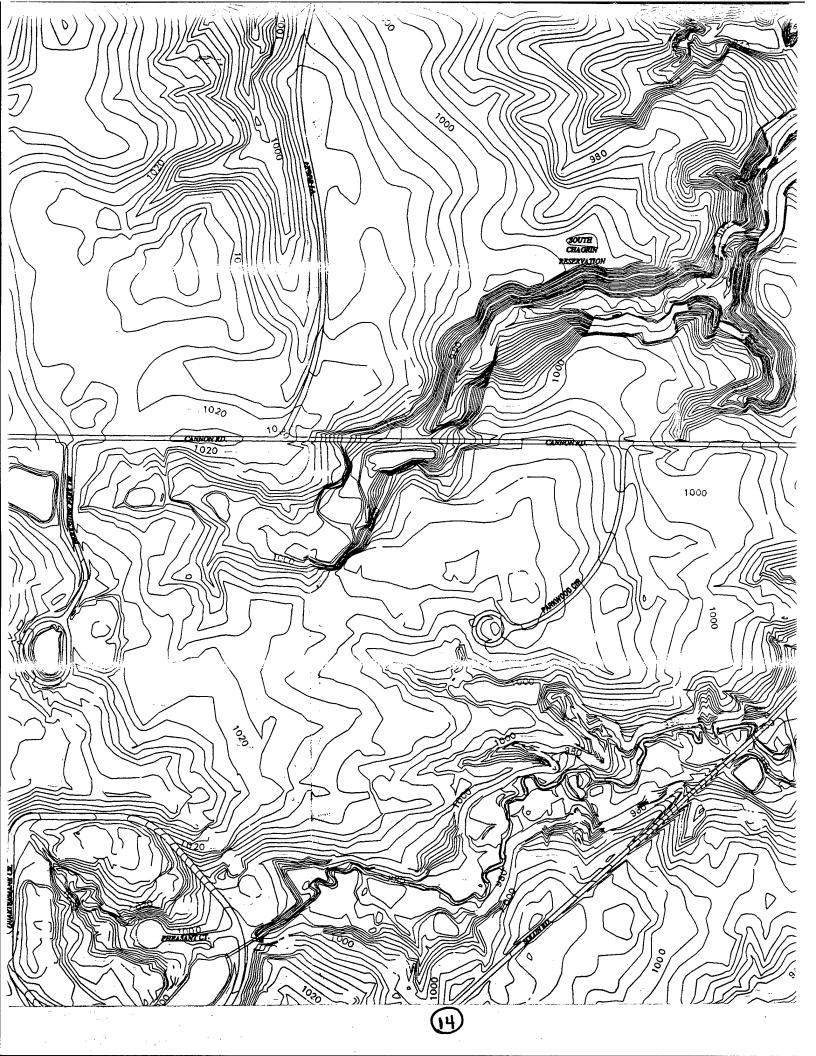


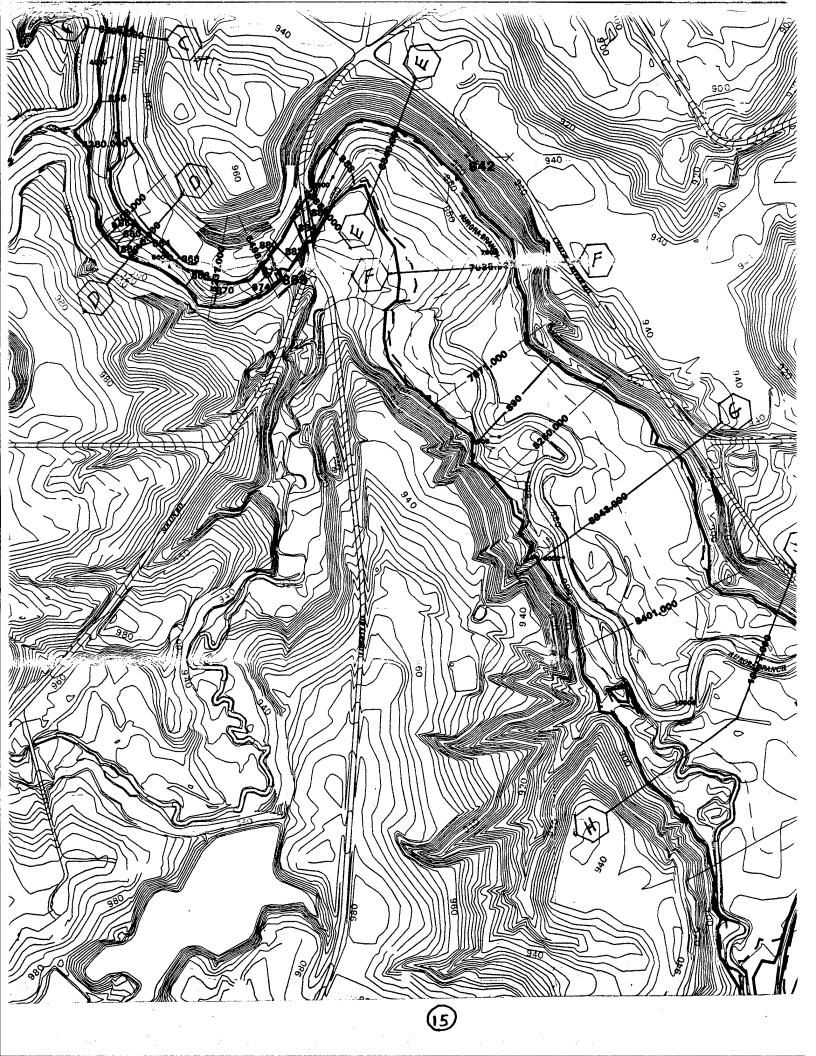
Railroads

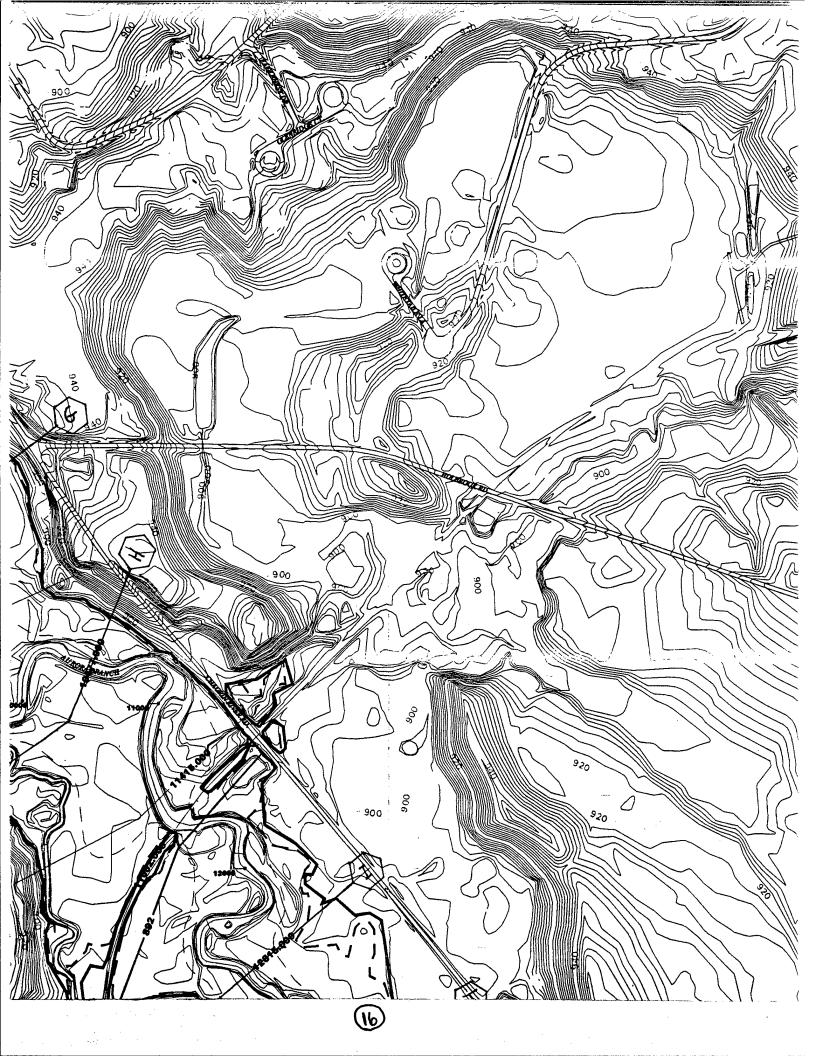


Political













Railroads



Political Boundary



Model Limits



Area Not Included



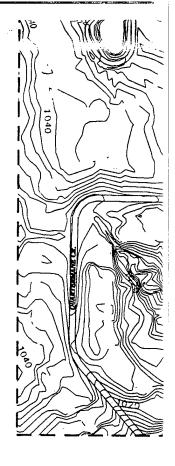
2 ft Contours



20 ft Contours



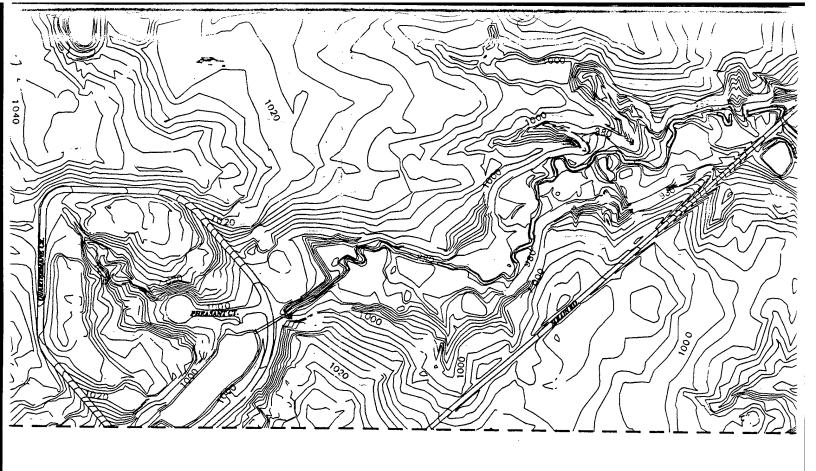
ERM



SPECIAL FLOOD HAZARD STUDY BENTLEYVILLE, OHIO

CHAGRIN RIVER AURORA BRANCH

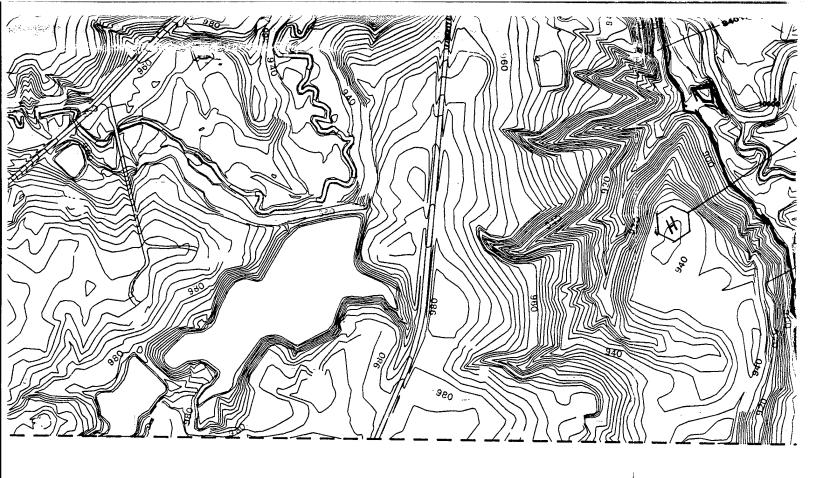




UDY

Base Mappi

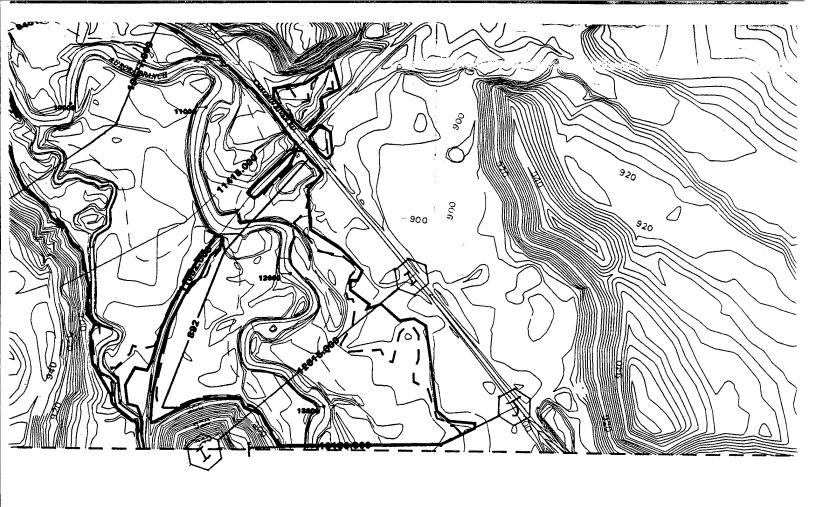
U



Mapping: CUYAHOGA COUNTY ENGINEER



US ARMY CORPS OF ENGINEERS BUFFALO DISTRICT



(aa)

Scale 1" = 400"

Horz. Datum: NGVD27

Vert. Datum: NGVD 1929

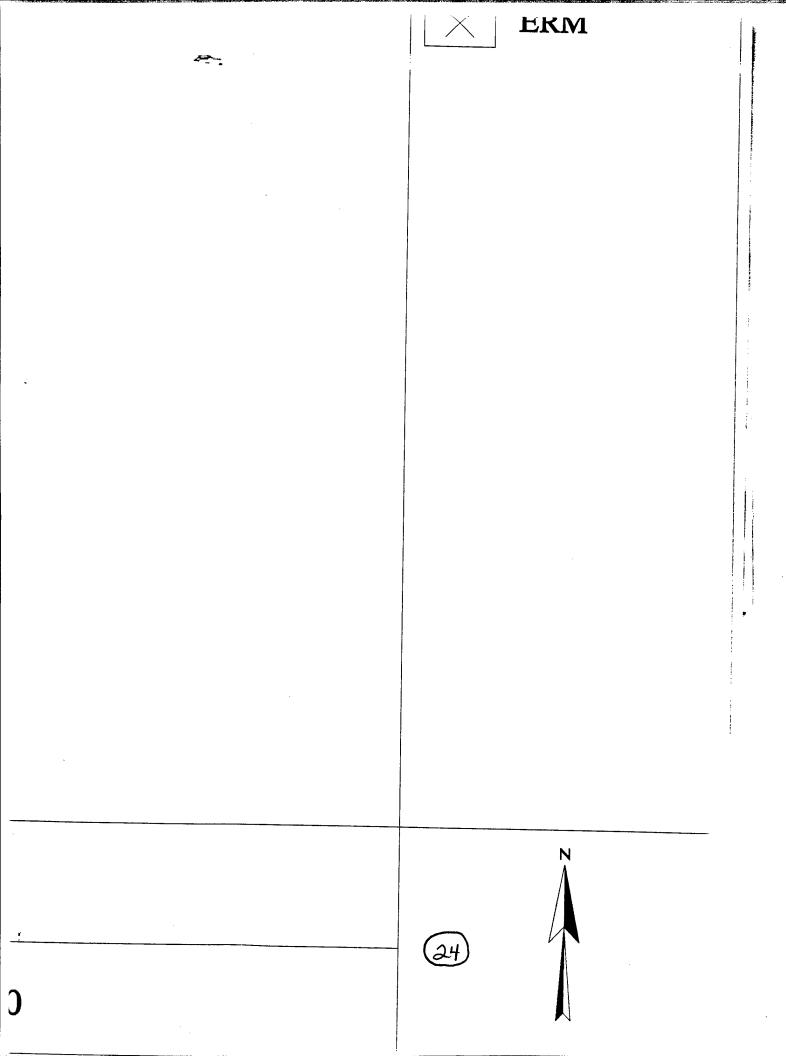


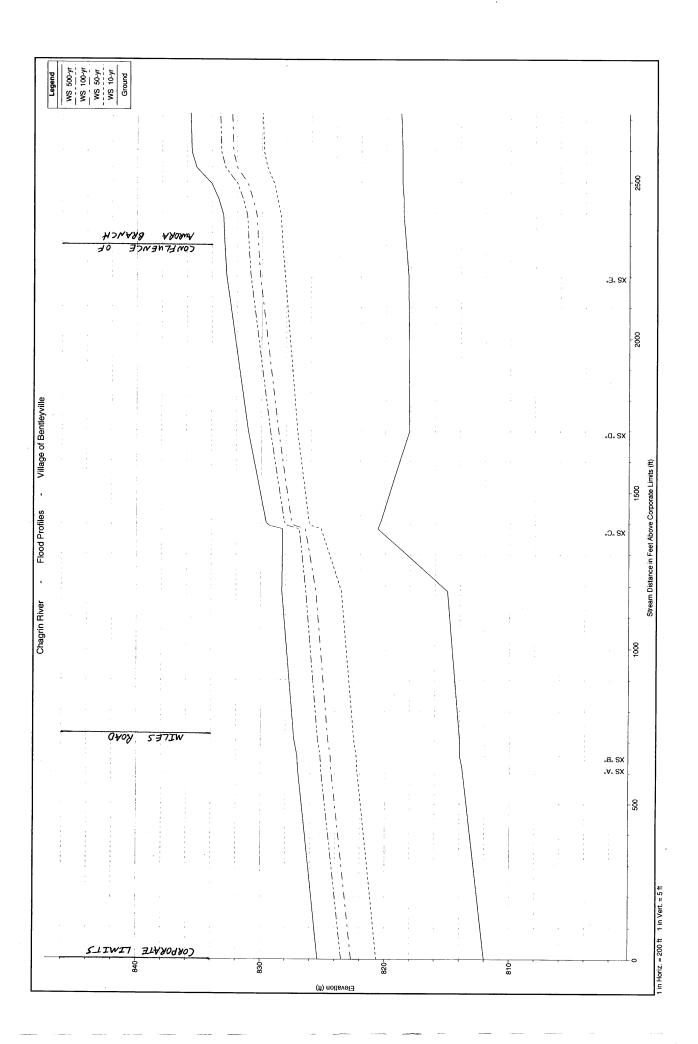
Control Grid: CRGS

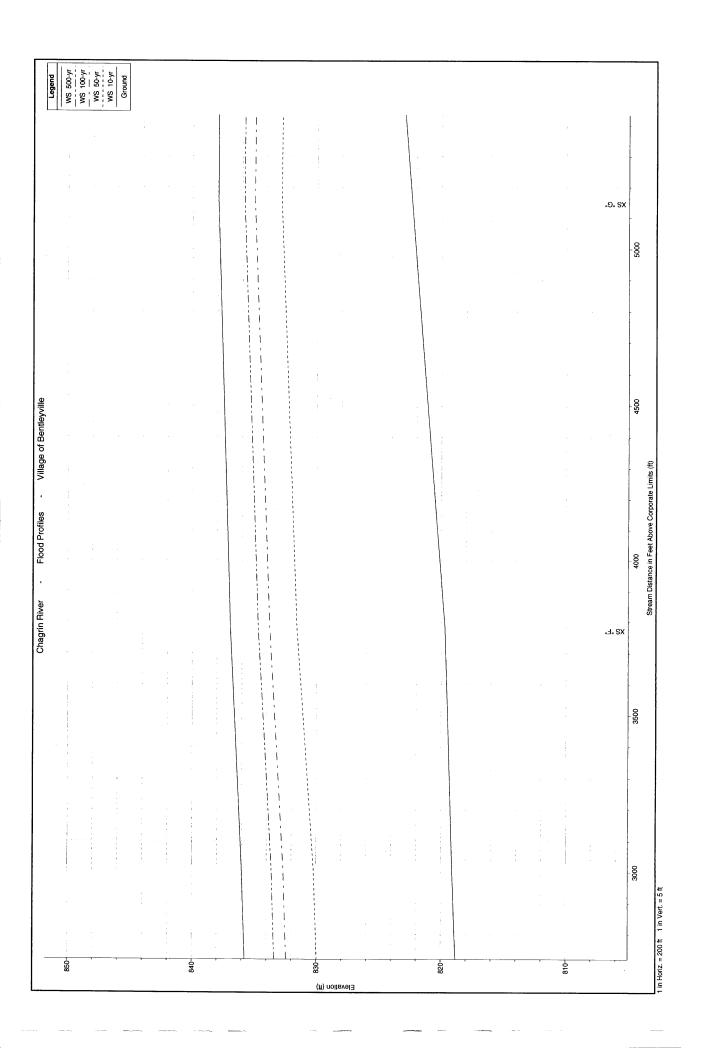
Q3

Date: March 21, 2000

129







(a) (a) (b) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c		一个一个一个一个一个一个一个一个一个一个一个一个一个一个一个一个一个一个一个					Ledend
32 (2) (2) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4							WS 500-yr
1.1 SX 1.2 SX 1.0 SX	:					>	VS 100-yr
11.5X A1.5X ON	-098			5]1		 	WS 50-yr
22 (2) (2) (3) (3) (3) (3) (3) (3) (3) (3) (3) (3				WIT			Ground
22/V05/V0) A. S.				, 3			
1.5X				31V			
1. CX				Por			
A. SX 092 A. SX 093		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1		<u>רסע</u>			
L.SX A.L.SX Sign Sign							
.I. SX							
.H. SX	840-	** ** ** ** ** ** ** ** ** ** ** ** **					-
TH. 2X 1990							
TT 2X TO 100 100 100 100 100 100 100 100 100 10							
H- SX 009 009 009 009 009 009 009 009 009 00							
XS-TT							
71-72 X X X X X X X X X X X X X X X X X X X	-1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
2500 6000 7700 7700 7700 7700 7700 7700 7	(jj) u						
71-7X 2009	vatio						
T. XS "T.		The second distribution of the second distributi					
TT 2X 0000 0000							
.H. 252 H. 2500 6600 6600 7500 7500							
14. 2X 0009							
TH' 2X XS TI. *** TH' 2X XS TI. ** TH' 2X XS TI. *** TH' 2X XS TI. ** TH' 2X XS TI. *** TH' 2X XS TI. ** TH' 2X XS TI. *** TH' 2X XS TI. *							

H. SX 0009 0099							
**** X X X X X X X X X X X X X X X X X	820	The second secon					
H SX 5500 6500 7500							
SSO 6500 7500							
SX S S S S S S S S S S S S S S S S S S							
X X X X X X X X X X X X X X X X X X X							
F. T. XX XS							
SX S	<u>, -</u>						
XX 0009 0009	810-	"H. S		"I" S			
005/ 000/ 000/ 000	2009	SX				=	
	0000	0009	0099	7000	7500	8000	

